

SKEW: A 45° Skew is detailed on this sheet. Details for other skews are similar.

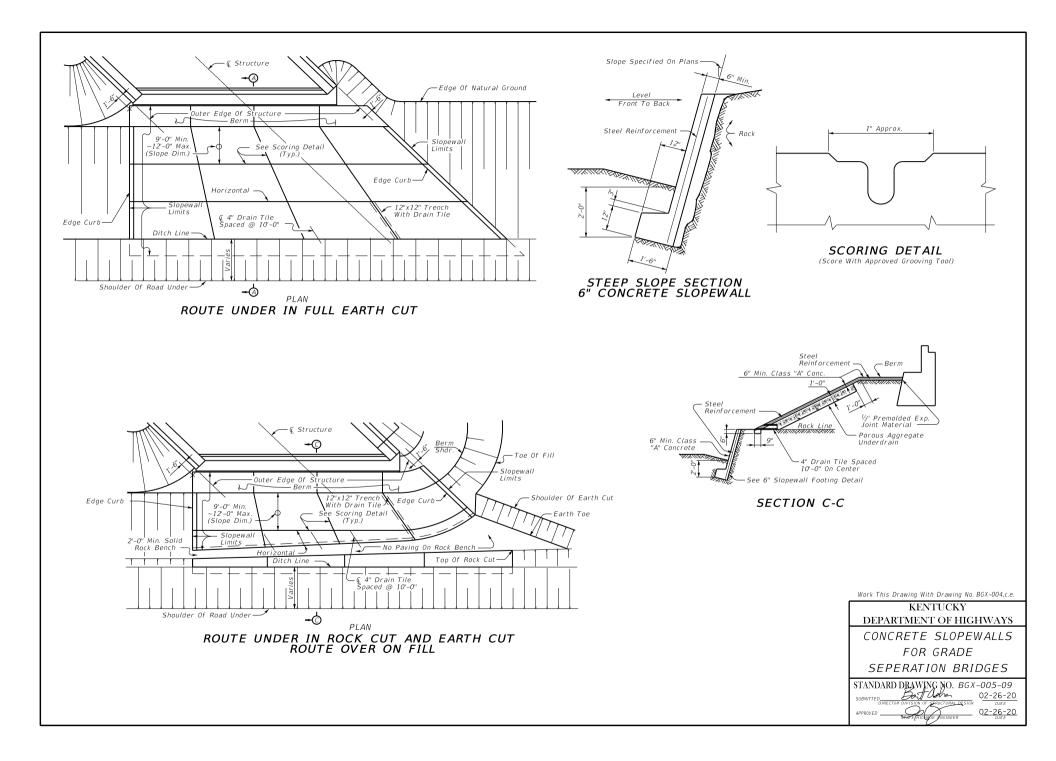
in each direction or an equivalent area of welded deformed steel fabric to reinforce the slopewall.

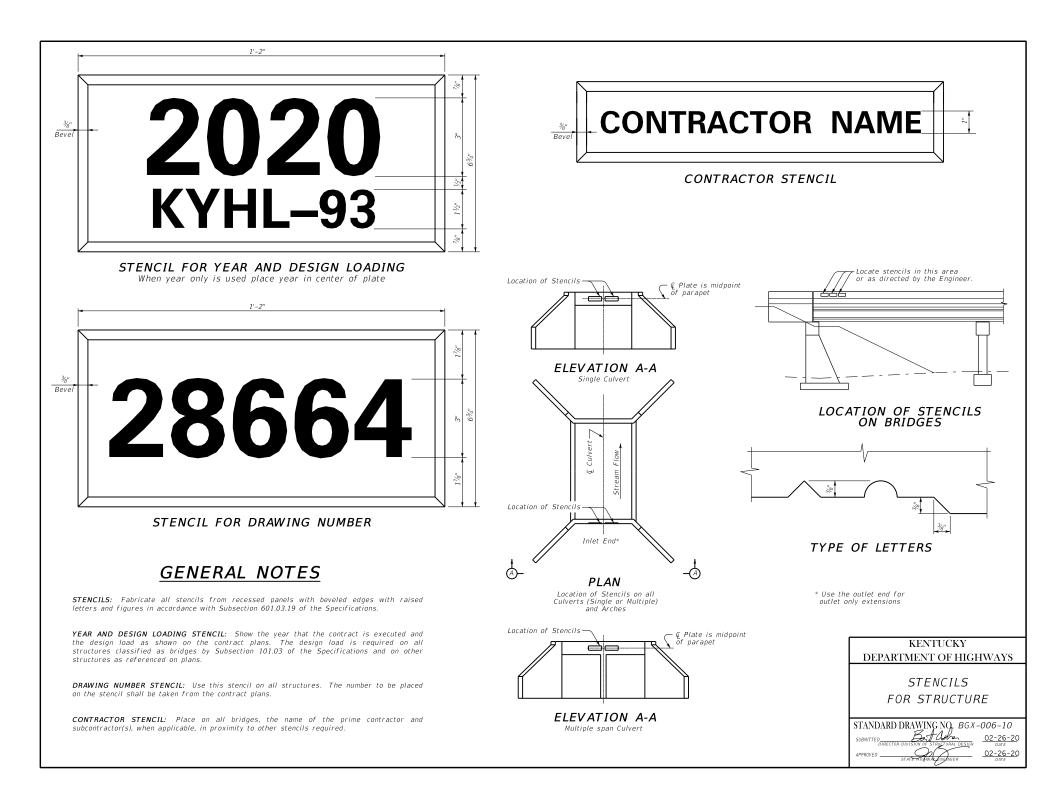
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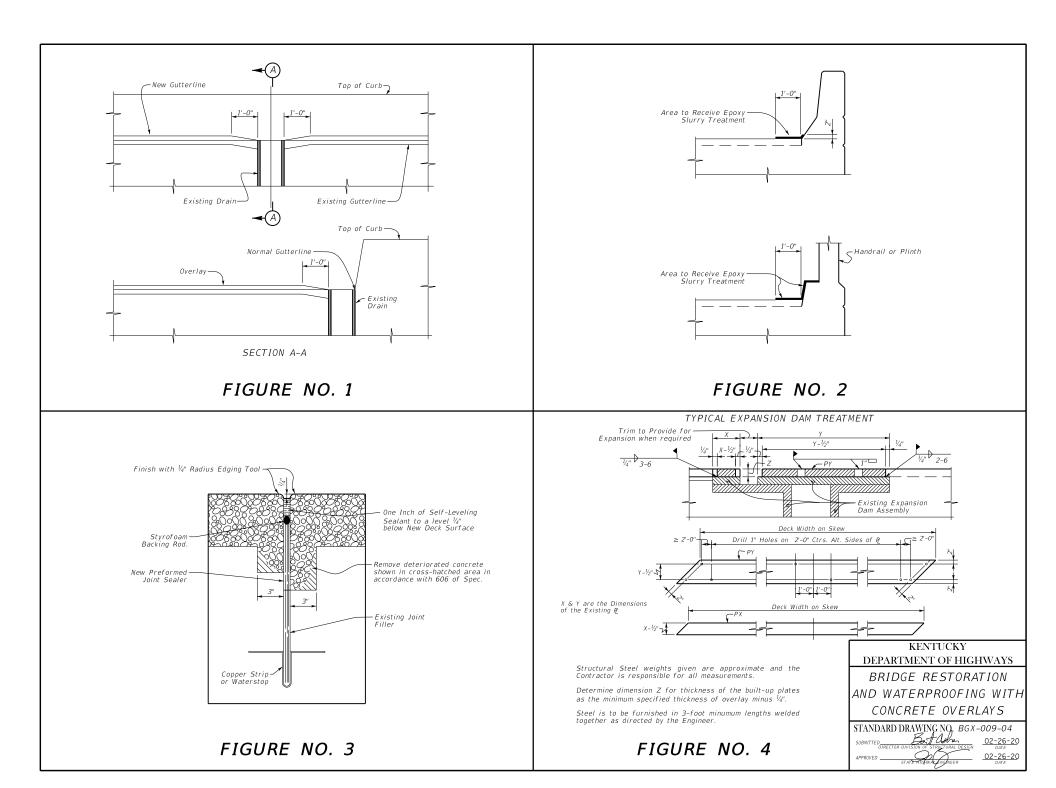
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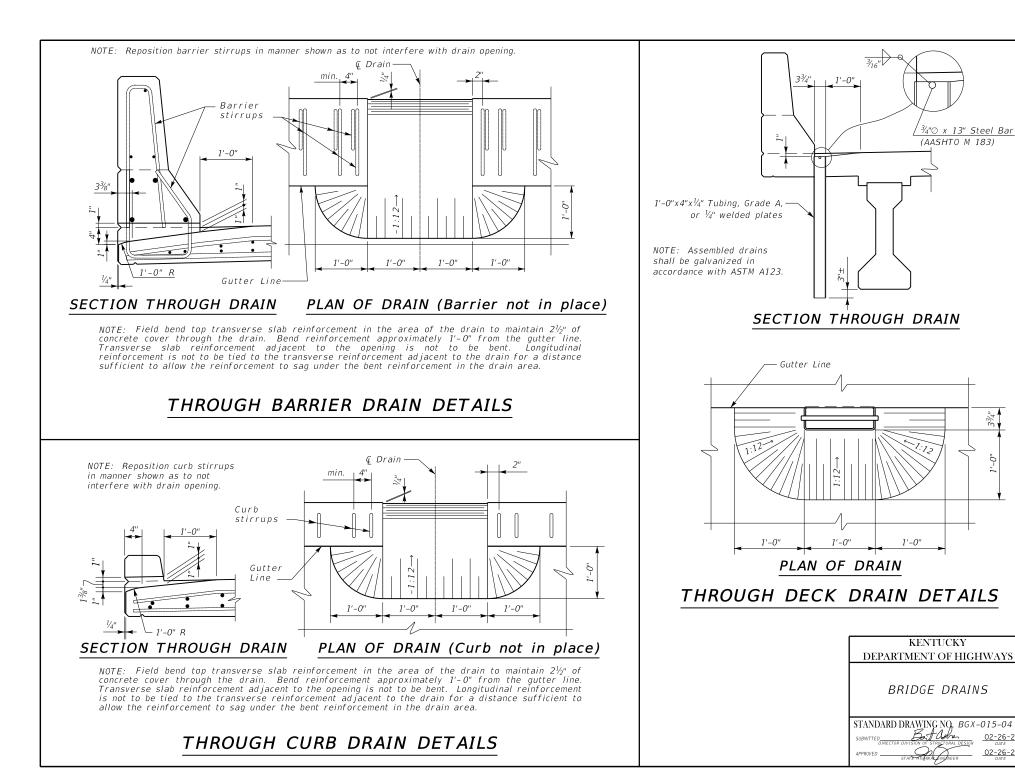
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Description of Soil Compactness or Consistency								
SOIL TYPE		COMPACTNESS OF CONSISTENCY	RANGE OF PENETRATION RESISTANCE	RANGE OF UNCONFINED COMPRESSIVE STRENGTH	AI LI N	Activity Index Liquidity Index Penetration Resistance		LIMESTONE
Coarse grained soils (More than half of material is larger than No. 200 sieve size.)		Very loose Loose Medium compact Compact Very compact	Less than 4 blows per ft. 4 to 10 10 to 30 30 to 50 Greater than 50	Not applicable	S+C(%) ○ ⊕ ⊚	Material finer than No. 200 sieve Rockline Soundings Disturbed Sample Boring Undisturbed Sample Boring		SANDSTONE
Fine grained soils (More than half of material is smaller than No. 200 sieve size.)		Very soft Soft Medium stiff Stiff Very stiff Hard	Not applicable	Less than 0.25 tsf 0.25 to 0.50 0.50 to 1.0 1.0 to 2.0 2.0 to 4.0 Greater than 4.0	© ¢ Â	Undisturbed Sample Boring & Rock Core Rock Core Slope Inclinometer Installation typical applications: ・		COAL
					0₩ ☞ □ <	7-Day (or greater) Water Table & Date Thin-walled Tube Sample Standard Penetration Test Sample	) )   ) )	NONDURABLE SHALE
			ssification		UU (psi)	Unconsolidated, Undrained Triaxial Test	_	(SDI < 90)
MAJOR	DIVISION	SYMBOL	NAI		Qu (psi) w (%)	Unconfined Compressive Strength Moisture Content		DURABLE
COARSE GRAINED SOILS	GRAVEL AND GRAVELLY SOILS SAND AND SANDY SOILS		Well-graded gravels of mixtures, little or no f Poorly graded gravels	fines.	RQD (%) SDI (JS) Rec. (%)	Rock Quality Designation Slake Durability Index (Jar Slake Test)		SHALE (SDI ≥ 90)
		GP	mixtures, little or no fines.		Ø	Core Recovery Angle of Internal Friction	.0.7.	TALUS OR
		GM GM	Silty gravels, gravel-s	and-silt mixtures.	Ø c (psi) <u>⊂</u> (psi)	Effective Angle of Internal Friction Cohesion Effective Cohesion	$\left[ \begin{array}{cc} \cdot & \cdot & \cdot \\ \cdot & \cdot & \cdot \end{array} \right] M$	IINE WASTE OR FILL MATERIAL
		<i>a</i>	Clayey gravels, gravel		γ RDZ	Total Unit Weight Rock Disintegration Zone	F	ROADWAY FILL- GRANULAR
		SW	Well graded sands or little or no fines.	gravelly sands,	0B IB R	Overburden Bench Intermediate Bench Refusal		EMBANKMENT
			Poorly graded sands of little or no fines.	or gravelly sands,	NR VS (psi)	Refusal Not Encountered Field Vane Shear Strength	<	STRUCTURE GRANULAR
		5M + + +	Silty sands, sand-silt	mixtures.			✓ >	BACKFILL
		SC × × ×	Clayey sands, sand-cla				0.0	SLOPE PROTECTION
FINE GRAINED - SOILS	SILTS AND CLAYS LL IS LESS THAN 50	ML	Inorganic silts and ver flour, silty or clayey f silts with slight plasti	fine sands or clavey			°0°°	TROTLETION
			Inorganic clays of low gravelly clays, sandy o lean clays.		Re	lation of RQD and in situ Rock Quality RDQ (%) Rock Quality	DEPAR	KENTUCKY TMENT OF HIGHWAYS
	SILT S AND CLAY S	мн	Inorganic silts, micace fine sandy or silty so			90 - 100 Excellent 75 - 90 Good 50 - 75 Fair 25 - 50 Poor	G	EOTECHNICAL LEGEND
	LL IS GREATER THAN 50	сн	Inorganic clays of hig	h plasticity, fat clays.		0 - 25 Very Poor	STANDARD	DRAWING NO. BGX-012-02 Bat Jaka 02-26-20
UNCLASSIFIED MATERIAL NONE Non-classified mater pavement, slag, etc.)						DIRECTOR	R DIVISION OF STRUCTURAL <u>DESIGN</u> DATE STATE TIGHNAL ENGINEER <u>02-26-20</u> DATE	

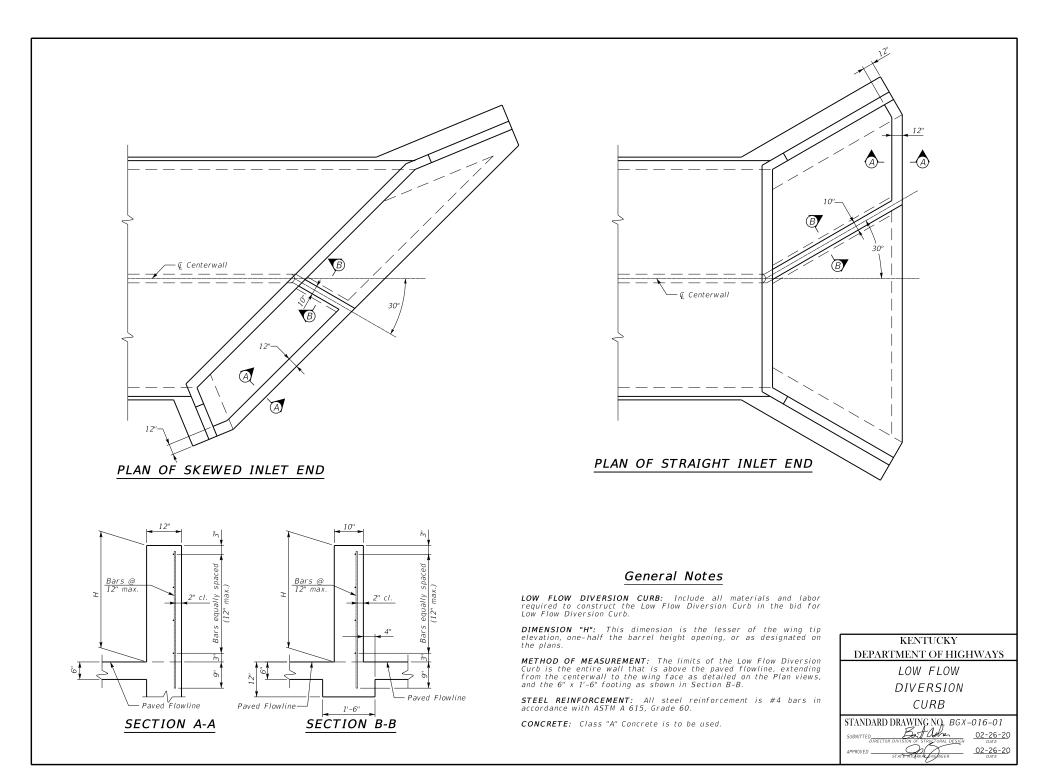


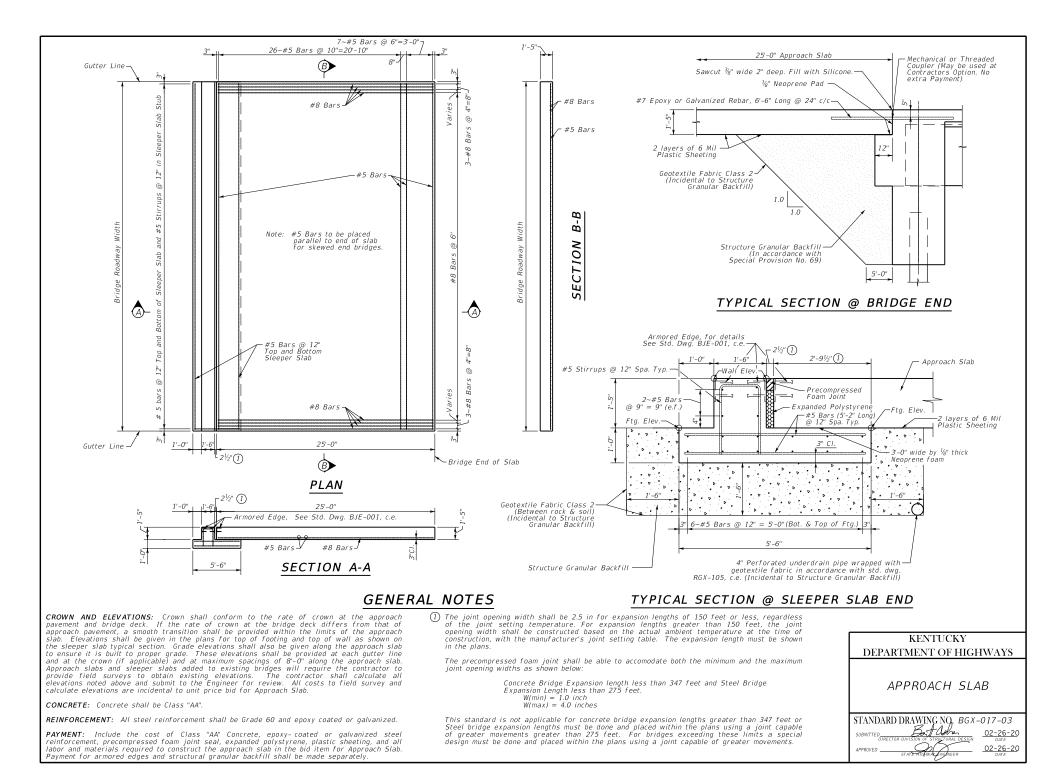
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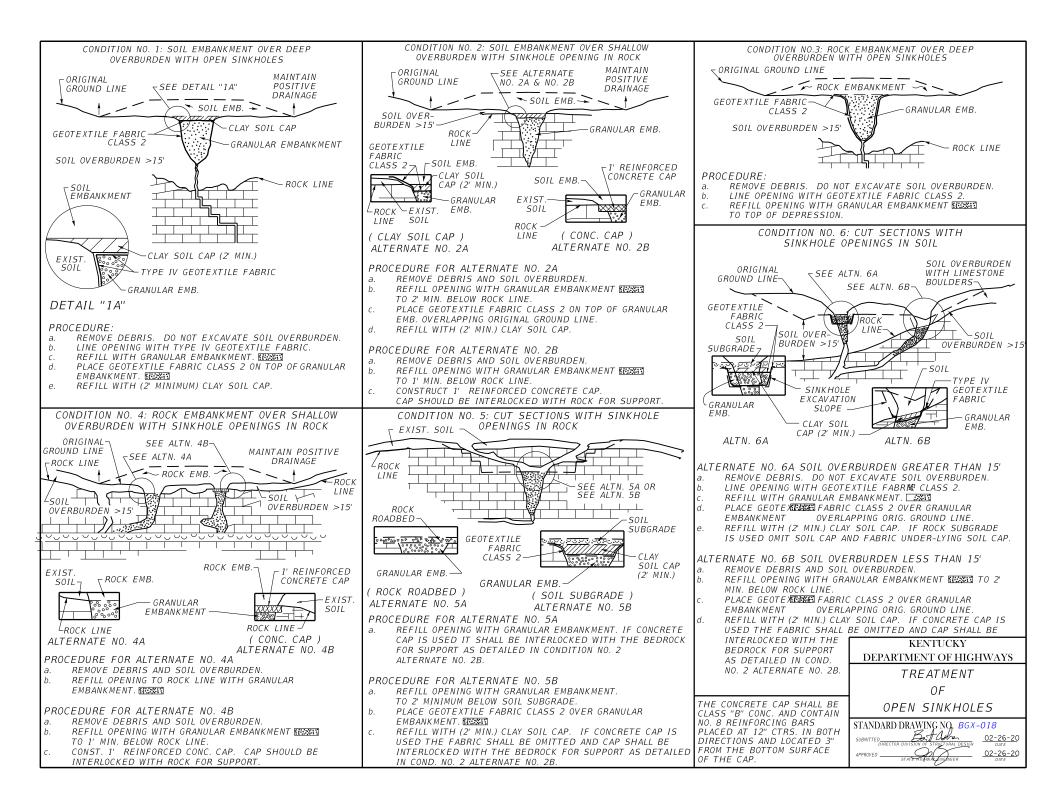
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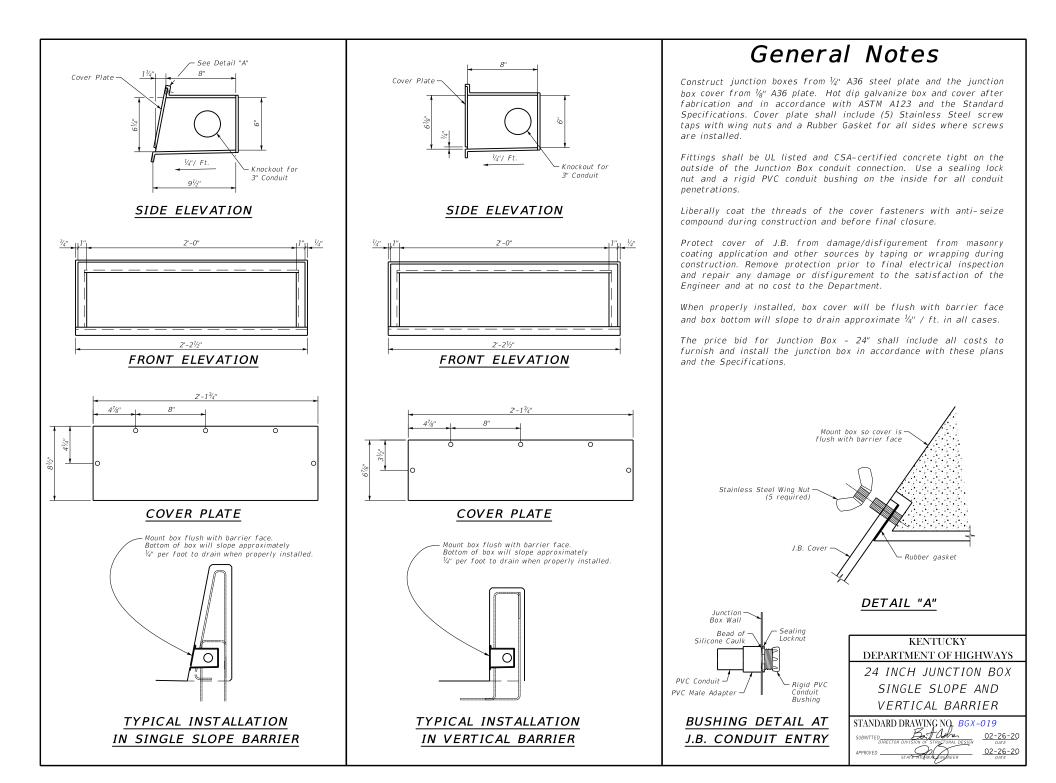
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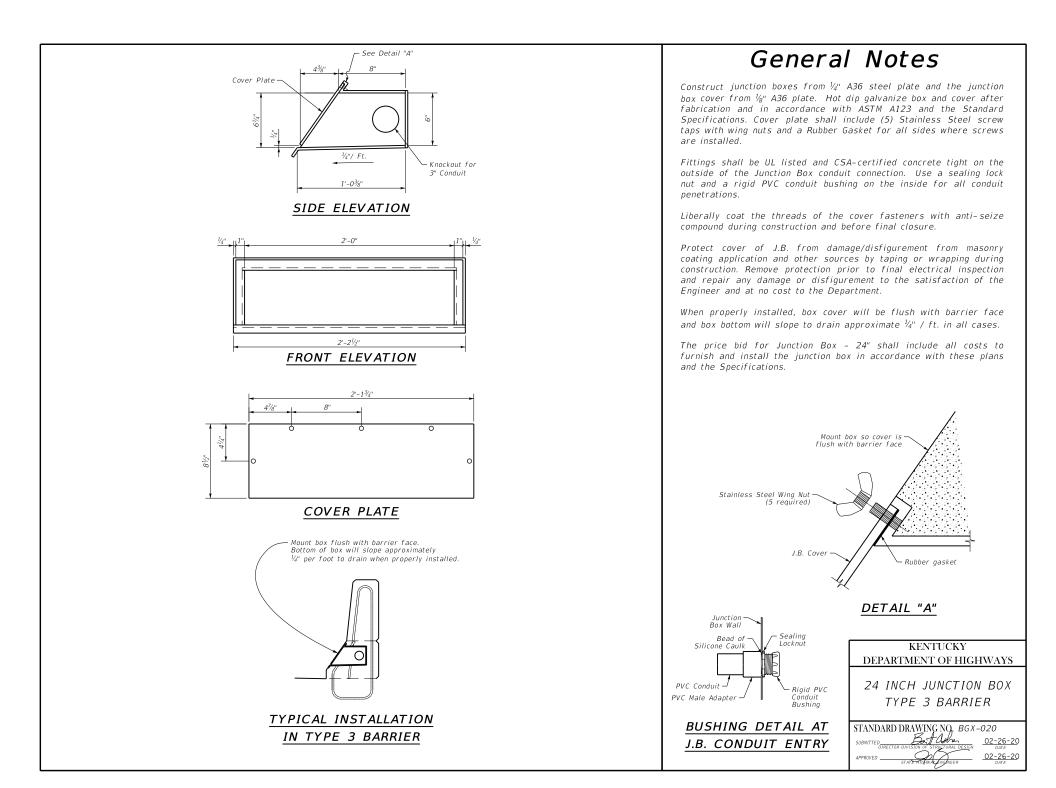
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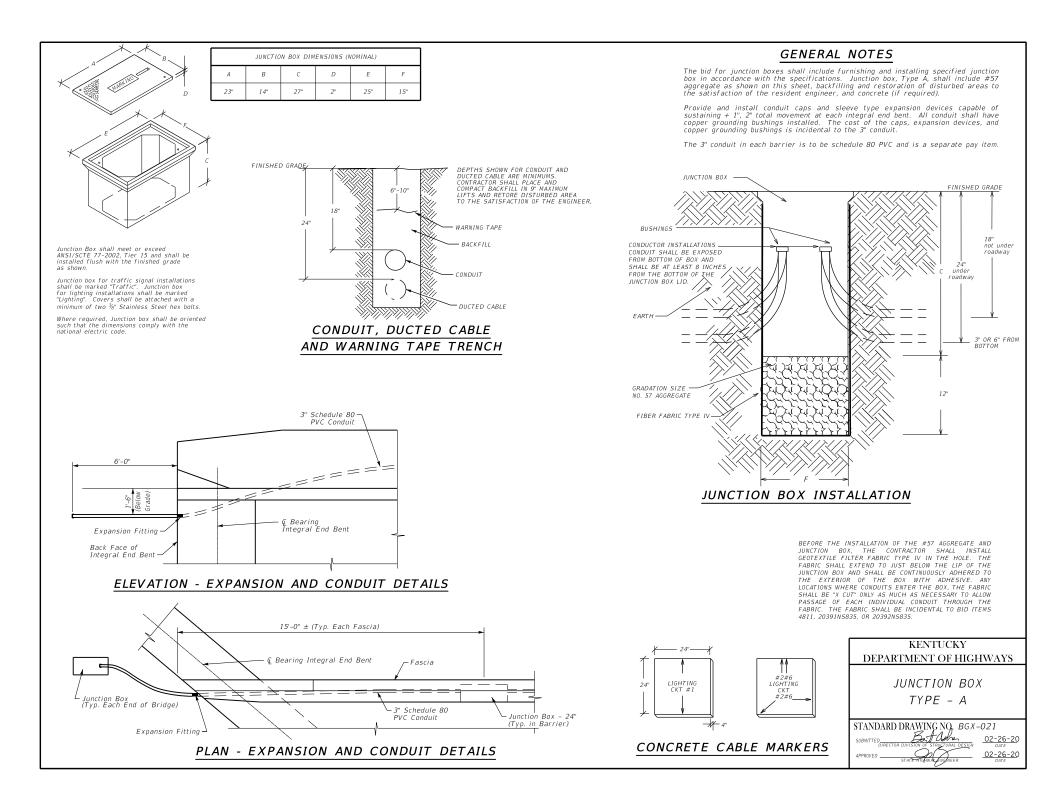


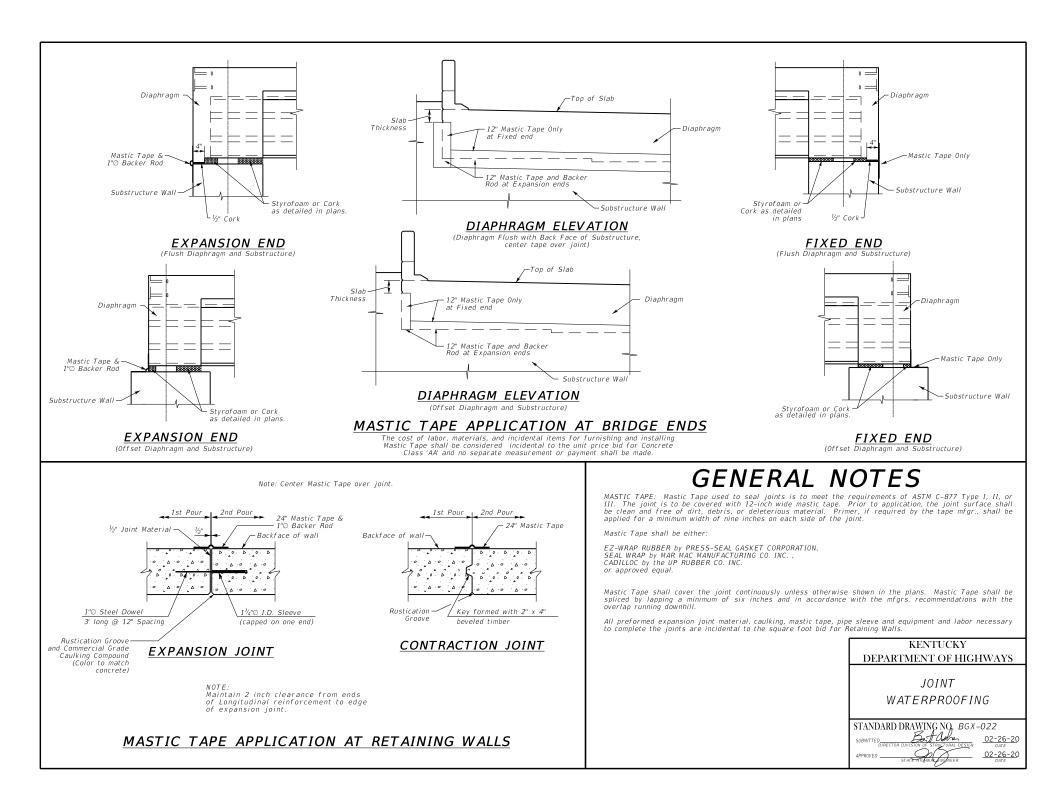


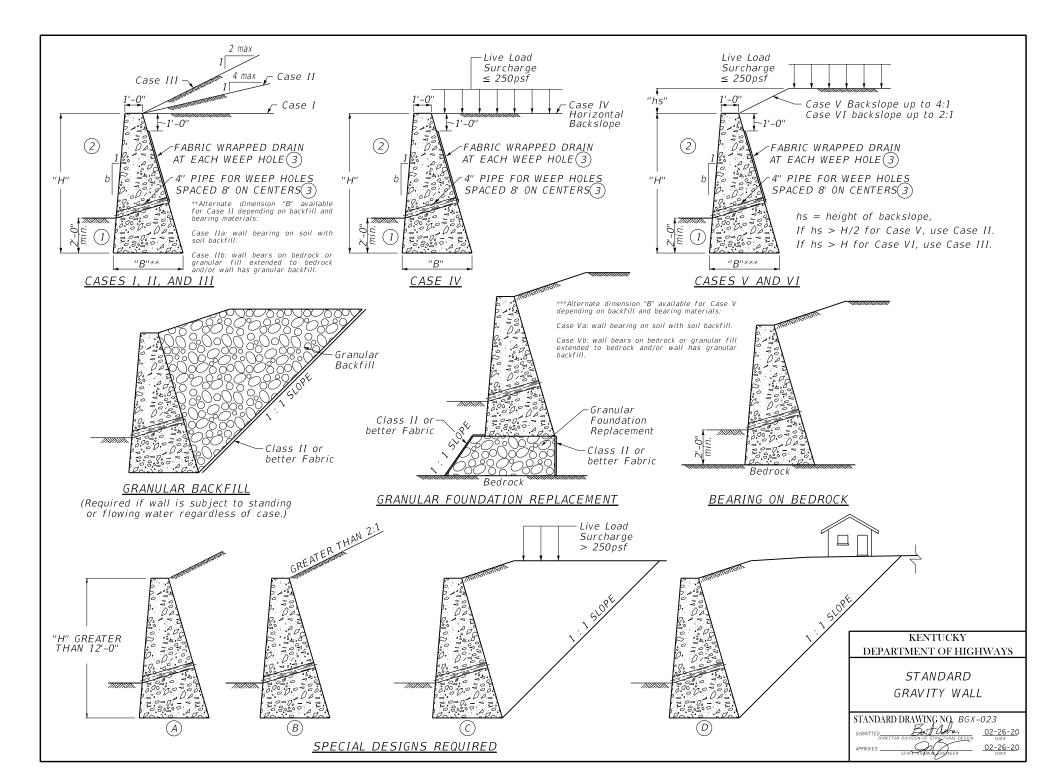












## STANDARD GRAVITY WALL NOTES

The retaining walls depicted on these drawings shall be used when the height (H) of the wall is 12'-0" or less provided the following conditions are met:

- CASE I Wall backfill slopes down, is level, or slopes up from wall at 20H:1V or flatter slope. This low slope allows for backfills that would be level except for the slope required to facilitate proper drainage.
- CASE II Backfill slopes steeper than 20H:1V but no more than 4H:1V.
- CASE III -Backfill slopes steeper than 4H:1V but no more than 2H:1V.
- CASE IV Backfill slopes down, is level, or slopes up from wall at 20H:1V or flatter slope (as needed to facilitate proper drainage) and has a maximum live load surcharge of 250 pounds per square foot applied behind the wall.
- CASE V Broken-back backfill slopes no more than 4H:IV to a level surface level (except for slope needed to facilitate proper drainage) and has a maximum live load surcharge of 250 pounds per square foot applied to the level portion of the backfill. If the height of the backfill to the slope break equals or exceeds one-half the height of the wall (hs ≥ H/2), use CASE II.
- CASE VI Broken-back backfill slopes up steeper than 4H:1V but no more than 2H:1V to a level surface (except for slope needed to facilitate proper drainage) and has a maximum live load surcharge of 250 pounds per square foot applied to the level portion of the backfill. If the height of the backfill to the slope break equals or exceeds the height of the wall (hs ≥ H), use Case III.

Special Designs shall be required when the following conditions exist:

- (A) Wall height is greater than 12'-0".
- (B) Backfill slopes are steeper than 2H:1V.
- C The wall is surcharged with a live load exceeding 250 pounds per square foot within the limits of a 1:1 slope extending from the base of the wall.
- D The wall is surcharged with a dead load (i.e., buildings, structures, or other permanent facilities) within the limits of a 1:1 slope extending from the base of the wall.
- (1) Minimum embedment value is 2'-0" for all cases.
- 2 Batter (b) shall be as follows: Cases I, II a, and II b - For H < 10'-0" use 12V:1H. For H => 10'-0" use 6V:1H. Case III - For H < 7'-0" use 12V:1H. For H=> 7'-0" use 6V:1H. Cases IV, V a, V b, and VI - For H < 6'-0" use 12V:1H. For H=> 6'-0" use 6V:1H.
- 3 Fabric wrapped drains and 4" pipe for weep holes shall be included in the unit price bid for gravity type retaining walls.

Granular backfill, granular foundation replacement to bedrock, or a wall bearing directly on competent un-weathered bedrock is required for the following cases:

Case II b for H > 9.5'Case III - for H > 8'Case V b for H > 10.5'Case VI for H > 9'

Walls subject to standing or flowing water (adjacent to streams, ponds, lakes, rivers, detention basins, etc.) shall have granular backfill meeting the requirements below regardless of the Case.

Granular backfill or granular foundation replacement to bedrock (when required) shall meet the requirements of "Granular Embankment" in Section 805 of the Standard Specifications, current edition, except that the maximum size is 4 inches with a minus No. 200 content not exceeding 5.0 percent. Use material that classified as nonerodible, as defined in Section 805 of the Standard Specifications, current edition. Gravels or sands, crushed or uncrushed, shall not be allowed. Place Class II or better Geotextile Fabric in accordance with Sections 214 and 843 of the Standard Specifications, current edition, as shown below, where there is a soil-granular material interface.

Construct standard gravity retaining walls according to Section 613 of the Standard Specifications, current edition, with the exceptions that the wall shall be constructed according to this Standard Drawing and that the wall's base width shall exceed  $\frac{1}{2}$  vertical height when required by plan notes or this Standard Drawing. Gravity walls meeting the criteria for Standard Gravity Wall defined herein may still require a Geotechnical Investigation. Conditions warranting a site investigation and geotechnical exploration shall be as defined in this Standard Drawing and KYTC Geotechnical Policy Manual Section 400, current edition.

		Required base width, B(ft)							1	
	H(ft)	Case	Case	Case	Case	Case	Case	Case	Case	1
	$\Pi(IL)$	Ι	Па	II b	III	IV	Vа	Vb	VI	1
	3.0	1.50	1.50	1.50	1.50	3.00	3.00	3.00	3.25	1
	3.5	1.75	1.75	1.75	1.75	3.25	3.25	3.25	3.50	1
	4.0	2.00	2.00	2.00	2.00	3.50	3.50	3.50	3.75	1
	4.5	2.25	2.25	2.25	2.25	3.75	4.00	4.00	4.25	1
	5.0	2.50	2.50	2.50	2.50	4.00	4.25	4.25	4.50	1
	5.5	2.75	2.75	2.75	2.75	4.25	4.50	4.50	4.75	12:1 Batter
	6.0	3.00	3.00	3.00	3.00	4.50	4.75	4.75	5.00	6:1 Batter
	6.5	3.25	3.25	3.25	3.25	4.75	5.00	5.00	5.50	1
	7.0	3.50	3.50	3.50	3.50	5.00	5.50	5.50	5.75	1
	7.5	3.75	3.75	3.75	3.75	5.25	5.75	5.75	6.00	1
	8.0	4.00	4.00	4.00	4.00	5.75	6.00	6.00	6.50	1
	8.5	4.25	4.25	4.25	4.25*	6.00	6.25	6.25	6.75	1
	9.0	4.50	4.50	4.50	4.50*	6.25	6.50	6.50	7.00	1
12:1 Batter	9.5	4.75	4.75	4.75	4.75*	6.50	6.75	6.75	7.25*	1
6:1 Batter	10.0	5.00	5.25	5.00*	5.00*	6.75	7.00	7.00	7.50*	1
	10.5	5.25	5.50	5.25*	5.25*	7.00	7.25	7.25	7.75*	1
	11.0	5.50	5.75	5.50*	5.50*	7.25	7.75	7.50*	8.25*	1
	11.5	5.75	6.00	5.75*	5.75*	7.50	8.25	7.75*	8.50*	1
	12.0	6.00	6.25	6.00*	6.00*	7.75	8.75	8.00*	9.00*	ı.

\* Requires Granular Backfill, Granular Foundation replacement to bedrock, or bearing directly on competent unweathered bedrock.

## **DESIGN PARAMETERS**

Large block retaining walls without reinforced backfill are suitable alternatives to Standard Gravity Walls. Approved large block retaining wall suppliers can be found on the Structural Design web site. Contractors shall submit to the Engineer for review and approval a design for a large block wall alternative using the following design parameters unless site-specific geotechnical information is provided or required. The wall design shall be in accordance with the AASHTO Standard Specifications for Highway Bridges, current edition.

Soil Backfill	$c' = 0 \text{ psf}, \Phi' = 28^{\circ}, \gamma = 120 \text{ pcf}$
Foundation	$c = 1200 \text{ psf}, \Phi' = 0, \gamma = 120 \text{ pcf}$
Granular backfill or foundation replacement	$c' = 0, \Phi' = 38^{\circ}, \gamma = 115 \text{ pcf}$
Pay Items:	

Cu. Yd.

Cu. Yd.

Cu. Yd.

Sq. Yd.

Concrete, Class B Structure Excavation Granular Embankment (when required) Geotextile Fabric (when required)



